- 2
- 3
- 4
- .
- 5

Truncated Physical Model for Dynamic Sensor Networks with Applications in High-Resolution Mobile Sensing and BIGDATA

Thomas J. Matarazzo, S.M.ASCE¹; and Shamim N. Pakzad, A.M.ASCE²

Abstract: Historically, structural health monitoring (SHM) has relied on fixed sensors, which remain at specific locations in a structural 6 system throughout data collection. This paper introduces state-space approaches for processing data from sensor networks with time-variant 7 8 configurations, for which a novel truncated physical model (TPM) is proposed. The state-space model is a popular representation of the 9 second-order equation of motion for a multidegree of freedom (MDOF) system in first-order matrix form based on field measurements and system states. In this mathematical model, a spatially dense observation space on the physical structure dictates an equivalently large 10 11 modeling space, i.e., more total sensing nodes require a more complex dynamic model. Furthermore, such sensing nodes are expected to 12 coincide with state variable DOF. Thus, the model complexity of the underlying dynamic linear model depends on the spatial resolution of the 13 sensors during data acquisition. As sensor network technologies evolve and with increased use of innovative sensing techniques in practice, it 14 is desirable to decouple the size of the dynamic system model from the spatial grid applied through measurement. This paper defines a new data 15 class called dynamic sensor network (DSN) data, for efficiently storing sensor measurements from a very dense spatial grid (very many sensing nodes). Three exact mathematical models are developed to relate observed DSN data to the underlying structural system. Candidate models are 16 compared from a computational perspective and a truncated physical model (TPM) is presented as an efficient technique to process DSN data 17 18 while reducing the size of the state variable. The role of basis functions in the approximation of mode shape regression is also established. Two 19 examples are provided to demonstrate new applications of DSN that would otherwise be computationally prohibitive: high-resolution mobile sensing and BIGDATA processing. DOI: 10.1061/(ASCE)EM.1943-7889.0001022. © 2016 American Society of Civil Engineers. 20

21 Author keywords: Dynamic sensor networks; Mobile sensors; BIGDATA; High spatial resolution.

22 Introduction

23 Structural health monitoring (SHM) endeavors began as observa-24 tions of operational vibrations of long-span bridges as early as the 25 1930s (Carder 1937) with increasing participation through the 1960s (Vincent 1962). By the late 1970s, numerous modal identi-26 27 fication studies (Abdel-Ghaffar 1976; McLamore et al. 1971; Rainer and Selst 1976; Trifunac 1970) had established prom-28 29 ising results and provided the motivation for modern techniques. 30 Through recent advancements in data processing, storage, mobile 31 computing, and sensing technology, SHM techniques have evolved 32 into repeatable, sophisticated analyses, often embedding statistical 33 frameworks or using statistical tests for decision making (Andersen 34 et al. 1999; Dorvash et al. 2014b; Juang and Pappa 1984; Lei et al. 35 2003; Shahidi et al. 2015; Smyth et al. 2003). A glimpse of the 36 recent growth in system identification methods is particularly 37 evident through the comparison of Abdel-Ghaffar and Scanlan 38 (1985) and Pakzad and Fenves (2009)-two analyses of ambient 39 vibrations observed at the Golden Gate Bridge, separated by two 40 decades.

However, all past SHM efforts have had one common attribute: a reliance on fixed sensor networks during data collection and processing. This dependency restricts the spatial information within the observed data. For example, in system identification (SID), the spatial resolution of the mode shapes is dependent on the arrangement of the fixed sensors (Matarazzo and Pakzad 2015b). Despite numerous implementations of spatially dense sensor networks (Dorvash et al. 2014a; Inaudi and Glisic 2010; Pakzad et al. 2008; Shahidi et al. 2015; Zhu et al. 2012) once instrumented, each sensor has remained at its position throughout collection of a single data set.

In the context of this paper, a single data set is defined as a time series matrix of measured values to be processed simultaneously. Some studies have, in fact, recorded data with moving sensors; however, in such cases, either the data were split into several smaller data sets based on each sensor configuration and analyzed as fixed network data (Zhu et al. 2012) or spatial information (precise positions of the sensors) was either not measured or ignored (Cerda et al. 2012; Gonzalez et al. 2012; Lin and Yang 2005; McGetrick et al. 2009; Yang et al. 2004). Note in absence of the sensors' spatial information, the data set is not compatible with state-space approaches and a comprehensive system identification is not possible.

Moreover, for the exception of Matarazzo and Pakzad (2014, 2015b), SHM processing is currently limited to analyzing one fixed sensor network configuration at a time. Data from multiple sensor configurations must be split into multiple data sets and analyzed separately as in Zhu et al. (2012). To be clear, this is not intended to be a criticism on the direction of SHM; this is simply an exploration into a new frontier of sensing and data processing.

41

42

43

44

45

46

47

48

49

50

51

52

53

54

55

56

57

58

59

60

61

62

63

64

65

66

67

68

69

¹Dept. of Civil and Environmental Engineering, Lehigh Univ., 117 ATLSS Dr., PA 18015 (corresponding author). E-mail: thomasjmatarazzo@ gmail.com

²Associate Professor, Dept. of Civil and Environmental Engineering, Lehigh Univ., 117 ATLSS Dr., PA 18015. E-mail: pakzad@lehigh.edu

Note. This manuscript was submitted on May 14, 2015; approved on September 9, 2015 No Epub Date. Discussion period open until 0, 0; separate discussions must be submitted for individual papers. This paper is part of the *Journal of Engineering Mechanics*, © ASCE, ISSN 0733-9399.

71 The development and implementation of new sensor technolo-72 gies as well as the techniques for processing new forms of data 73 sets efficiently are motivated by both an improvement in extractable 74 structural information and a reduction in network setup efforts. 75 Subsequent new data classes often have inherently different proper-76 ties in comparison to typical fixed sensor data, which dominate 77 SHM today, and create unique processing challenges, e.g., fusion 78 of data sampled at different rates (Smyth and Wu 2007), data 79 with missing observations or data from mobile sensors networks 80 (Matarazzo and Pakzad 2015b; Matarazzo et al. 2015b), or 81 prohibitively large data dimensions of BIGDATA (Matarazzo et al. 82 2015a).

This paper proposes and examines dynamic sensor network 83 84 data. In brief, data from a dynamic sensor network (DSN) are 85 an amalgamation of measurements from numerous sensing con-86 figurations. The merit of DSN data is its high capacity for storing spatial information; measurements from a very large quantity of 87 88 sensing nodes can be condensed into a much smaller matrix. 89 For example, high-resolution mobile sensor networks or BIGDATA 90 are efficiently represented in DSN data.

This paper is organized as follows. The section "Dynamic 91 92 Sensor Network Data" defines fundamental properties of DSN and 93 corresponding DSN data sets. The section "Exact State-Space Models for Dynamic Sensor Networks" presents two state-space 94 95 models that have been suited for processing DSN data and intro-96 duces the truncated physical model (TPM) as an efficient model for 97 processing data of this class. The section "Mode Shape Regression 98 Using Basis Functions" discusses the use of the sinc and spline 99 basis functions for approximating the mode shape regression term, 100 which is included in the state-space models considered. The section 101 "Processing Data from Novel Sensing Techniques" utilizes the pro-102 posed TPM for two novel sensing techniques: high-resolution 103 mobile sensing and BIGDATA. Finally, the challenges in process-104 ing DSN data are summarized, the advantages of the TPM are 105 reviewed, and a catalog of the nomenclature used among the 106 state-space models is provided.

107 Dynamic Sensor Network Data

108 This section introduces the concept of a dynamic sensor network (DSN) and the form of its corresponding DSN data. The dynamic 109 110 nature of DSN is well exemplified by a network of sensors that physically move in space while recording data in time. In this 111 case, each sensor channel is a time series from various points in 112 space, and when concatenated, the sensor channels form a DSN 113 data matrix. It is fundamental that the coordinates of each sensor 114 are known for every sample. Assume sensor locations are stored in 115 a sensor-position matrix. Through use of this sensor-position 116 matrix, the DSN data entries, which are mixed space-time mea-117 surements, can be decoded and properly included in a mathemati-118 119 cal model.

Spatial discontinuities are the definitive characteristic of DSN 120 121 data and are evident by inspection of the sensor-position matrix. In this case, sensing locations vary with time due to sensor mobility, 122 123 and in general, a time step in which the position of any sensor 124 changes indicates a spatial discontinuity in the DSN data matrix. 125 This paper focuses on analyzing DSN data in this form, i.e., as a single matrix, without splitting the data into configuration-based 126 pieces at spatial discontinuities. The remainder of this section 127 further defines properties of DSN, DSN data, and their applica-128 129 tions. In the following section, modeling approaches are proposed 130 to account for the spatial discontinuities present in DSN data.

Sensors, Sensing Nodes, and Observations

144

160

In fixed sensor networks, sensing nodes are exactly the points 132 where the sensors are installed. Typically, when these measure-133 ments are incorporated into the state-space model, the system states 134 (structural DOF) are, by default, assigned at these same sensing 135 nodes. In DSN, sensing nodes define the measurement space: the 136 spatial grid that contains the recorded sensor data. Therefore, in 137 DSN it is necessary to differentiate between these entities. For a 138 given DSN data matrix, let the observations be the total number 139 of columns N_O in the matrix, let the total number of sensing nodes 140 be *N*, and let the total number of sensors (measurement channels) 141 be N_{mc} . The ratios between these entities vary with each sensing 142 technique, but in general, N is a very large integer. 143

General Types of DSN

A physical DSN system is not required to obtain DSN data. There 145 are three general types of DSN data, each characterized by the 146 source of the inherent spatial discontinuities: online, offline, and 147 hybrid. Online DSN data come from a physical DSN, a time-148 varying sensor arrangement that records data, without pause, using 149 multiple sensing configurations (groups). In this case, switches be-150 tween groups are due to the physical movement of some (if not all) 151 sensors during data collection. Offline DSN data are extracted from 152 a fixed sensor network after data collection. For offline DSN, 153 nearly all data parameters, including sensor group sizes and group 154 switching schedules, are customized by the user after data collec-155 tion. Lastly, hybrid DSN data combine online and offline DSN; 156 sensing subgroups are extracted from a physical DSN, but after data 157 collection. The following two subsections consider an application 158 of online and offline DSN, respectively. 159

High-Resolution Mobile Sensing as an Online DSN

In high-resolution mobile sensing, relatively few moving sensors 161 scan a very large number of sensing nodes. A general illustration 162 of this form of online DSN is provided in Fig. 1, where a group of 163 three moving sensors collects N - 2 samples over N sensing nodes. 164 The sensing group moves at a constant velocity and shifts to a new 165 set of nodes after each sample, more specifically, each sensor 166 moves to the next node to the right. The constant physical obser-167 vation switching of this sampling mechanism causes spatial discon-168 tinuities in the DSN data matrix at every time step. Data collection 169

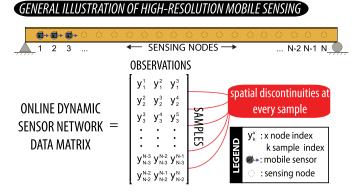
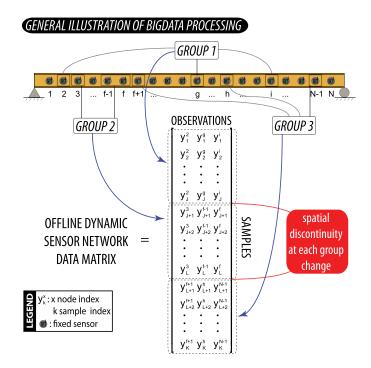


Fig. 1. General illustration of processing high-resolution mobile sensorF1:1data; three moving sensors simultaneously pass through N sensingF1:2nodes while sampling; the sensors move rightward in increments ofF1:3one node per sample with N - 2 samples in total; the correspondingF1:4online DSN data matrix is provided with spatial discontinuities at everyF1:5sampleF1:6



F2:1 **Fig. 2.** General illustration of processing BIGDATA using three observations and three sensing groups: group 1 consists of nodes 2, *g*, and *i*; group 2 consists of nodes 3, f - 1, and *f*; group 3 consists of nodes F2:4 f + 1, *h*, and N - 1; the corresponding offline DSN data with *K* total samples contains two spatial continuities, one at k = J + 1 and another F2:6 at k = L + 1

170 begins when all sensors are at sensing nodes on the left and ends 171 when all sensing nodes have been scanned, i.e., N - 2 samples in 172 total. In this case, the observation size N_O is equal to the number of 173 sensors N_{mc} , both of which are much smaller than the number of 174 sensing nodes N, i.e., $N_O = N_{mc}$ and $N_O, N_{mc} \ll N$.

175 Processing BIGDATA as Offline DSN Data

In one definition, BIGDATA refers to a very large data matrix con-176 taining samples from a very large number of sensors (equally many 177 178 sensing nodes), the result of a large-scale SHM endeavor. It is 179 not feasible nor in many cases is it necessary to process all of this BIGDATA simultaneously, if at all; even simple operations such as 180 181 uploading all measured data for processing could require significant computational efforts (Matarazzo et al. 2015a). A useful strat-182 183 egy is to extract an information-packed subset, an offline DSN data 184 set, from the BIGDATA population, i.e., a user-selected data matrix 185 in which a vast amount of spatial information is condensed into a 186 small size. A benefit of this approach is the high versatility of offline DSN data. Given BIGDATA, there are numerous potential 187 188 offline DSN data sets since the user has the ability to choose every 189 entry of the subset, which can be of any size (of course, not exceed-190 ing BIGDATA dimensions).

191 A general illustration of this type of offline DSN data is pro-192 vided in the example in Fig. 2 where three distinct sensing groups 193 form the data matrix: group 1 includes nodes 2, g, and i; group 2 194 includes nodes 3, f - 1, and f; group 3 includes nodes f + 1, h, and N-1. The $K \times N_O$ DSN data matrix contains spatial discon-195 tinuities at k = J + 1 and k = L + 1 corresponding with user-se-196 197 lected sensing groups. In this case, the observation size is three 198 $(N_O = 3)$, which is much smaller than the number of fixed sensors 199 instrumented at the sensing nodes, i.e., $N_O \ll N_{mc}$ and $N_{mc} = N$.

Exact State-Space Models for Dynamic Sensor Networks

This section introduces exact state-space models in which underlying state DOF responses are mapped from DSN data. The following subsections present three state-space models that represent a structural system exactly and have been tailored to incorporate DSN data properly. The sizes of these models and their corresponding efficiencies are discussed and compared. The first two subsections present modified versions of the familiar standard and modal state-space models that simultaneously consider a small number of observations (N_O data columns) and a large number of sensing nodes (N locations); in these situations, the benefits of DSN data are most evident. The adjustments to these models have a physical significance: they relate the structural response at one location to the response at another. Despite their similar mathematical forms, each model has distinct attributes and challenges.

In the third subsection, the truncated physical model (TPM) is introduced as an efficient solution for modeling DSN observations. In this context, an efficient model remains exact and requires minimal computational efforts; this is dictated by the sizes of model parameters (matrices), which are dependent on the definitions of the state variable and the observations.

Consider the second-order continuous-time equation of motion for a linear N-DOF system, where N is a very large integer

$$\bar{m}\ddot{\mathbf{u}}(t) + \bar{c}\dot{\mathbf{u}}(t) + \bar{k}\mathbf{u}(t) = B_f \mathbf{\eta}(t) \tag{1}$$

The locations of the N lumped masses are defined by the spatial 225 vector $\mathbf{s} = [s_1 \ s_2 \ \dots \ s_N]^T$. Note there is a DOF at every 226 sensing node, i.e., they are coincident. Sampled structural re-227 sponses (at sampling rate $f_s = 1/\Delta t$) are available at all DOF 228 via the full spatial vector s. Similarly, a general subset of these 229 DOF, called s_i , is comprised of some elements in s, i.e., $s_i \subset s$, 230 and refers to responses at selected DOF. Various spatial subvectors 231 of this form will be introduced to reference specific DOF subsets, as 232 opposed to all N DOF at once. The structural responses considered 233 are defined for time steps k = 1, 2, ..., K: where $\mathbf{u}_k(\mathbf{s}_i)$ is a vector 234 of displacements at DOF defined by \mathbf{s}_i at time step k; $\dot{\mathbf{u}}_k(\mathbf{s}_i)$ is a 235 vector of velocities at DOF defined by \mathbf{s}_i at time step k; and $\ddot{\mathbf{u}}_k(\mathbf{s}_i)$ 236 is a vector of accelerations at DOF defined by \mathbf{s}_i at time step k. 237

In this section, the standard state-space model, modal state-238 space model, and truncated physical model (TPM) are formulated 239 with the objective of using field measurements (observations) at N_O 240 sensing nodes defined by $s_O \subset s$ to describe the behavior of the 241 structural system through the state variable, e.g., \mathbf{x}_k . The observa-242 tion vector \mathbf{y}_k describes the exact responses at N_O DOF defined by 243 $\mathbf{s}_{0} \subset \mathbf{s}$ as shown in Eq. (2) and remains valid for all subsequent 244 state-space models 245

$$\mathbf{y}_k \equiv \ddot{\mathbf{u}}_k(\mathbf{s}_O) \tag{2}$$

Some final notes are necessary before the models are presented. 246 The primary difference between typical state-space models for 247 fixed sensor networks and those for dynamic sensor networks 248 (DSN) (which are presented in the following subsections) is that 249 the latter require the sensors' positions to be known precisely. 250 For DSN, the locations of the observations will be a function of 251 time step k, i.e., O = O(k). Consider \mathbf{s}_O as the kth row (transposed) 252 of a $K \times N_O$ sensor-position matrix, S_O , corresponding to the sen-253 sors in the DSN data matrix. For simplicity, this will not be explic-254 itly included in successive notation; however, in the following 255 subsections, model entities that depend on \mathbf{s}_{O} will also vary at every 256 time step, e.g., Φ_O . 257

200 201

202

203

204

205

206

207

208

209

210

211

212

213

214

215

216

217

218

219

220

221

222

223

Nomenclature tables are provided at the end of the paper for reference to model entities. Lastly, theoretically, the model orders for each subsequent state-space model, *p*, are all equal to 2; however, since significantly higher model orders are commonly considered in system identification applications (Chang and Pakzad 2013), a general definition is presented when referring to vector and matrix sizes.

265 Standard State-Space Model

This subsection presents the first state-space model under consideration for DSN: the standard state-space model. In this framework, the state vector \mathbf{x}_k , shown in Eq. (3), represents structural responses at all *N* DOF as defined by **s**

$$\mathbf{x}_{k} \equiv \begin{bmatrix} \mathbf{u}_{k}(\mathbf{s}) \\ \dot{\mathbf{u}}_{k}(\mathbf{s}) \end{bmatrix}$$
(3)

270 Also, given the full mode shape matrix $\Phi = \Phi^{\langle M \rangle}(\mathbf{s})$, where Φ is 271 an $N \times M$ matrix (inherently truncated to M = N modes due 272 to mass discretization), submode shapes matrices $\Phi_i = \Phi^{\langle M \rangle}(\mathbf{s}_i)$ 273 describe modal ordinates for respective spatial subvectors, 274 e.g., $\Phi_O = \Phi^{\langle M \rangle}(\mathbf{s}_O)$ is an $N_O \times M$ matrix. Eqs. (4) through (8) 275 provide discrete-time state-space model parameters

$$A_c \equiv \begin{bmatrix} 0 & I\\ -\bar{m}^{-1}\bar{k} & -\bar{m}^{-1}\bar{c} \end{bmatrix}$$
(4)

$$A = \exp(A_c \Delta t) \tag{5}$$

$$B_c \equiv \begin{bmatrix} 0\\ -\bar{m}^{-1}B_f \end{bmatrix} \tag{6}$$

$$B = A_c^{-1}(A - I)B_c \tag{7}$$

$$C \equiv C_a \begin{bmatrix} -\bar{m}^{-1}\bar{k} & -\bar{m}^{-1}\bar{c} \end{bmatrix}$$
(8)

276 Once the parameters are defined, the second-order differential 277 equation is expressed in first-order form through the state Eq. (9) 278 and the observation Eq. (10) for DSN

$$\mathbf{x}_k = A\mathbf{x}_{k-1} + B\mathbf{\eta}_{k-1} \tag{9}$$

$$\mathbf{y}_k = \Phi_O \Phi^{-1} C \mathbf{x}_k \tag{10}$$

Along the usual model parameters (Juang and Phan 2001), the 279 product $\Phi_0 \Phi^{-1}$ is added to the observation equation. This term rep-280 resents the regression of the responses at s on those at s_0 and is the 281 282 key to modeling the dynamics of one set of DOF while observing another; this entity is henceforth called the mode shape regression 283 284 (MSR) term. More specifically, the acceleration responses $\ddot{\mathbf{u}}_k(\mathbf{s}) =$ $C\mathbf{x}_k$ are first converted to modal coordinates through Φ^{-1} , then 285 reverted to physical coordinates using Φ_0 , finally representing 286 $\mathbf{y}_k = \ddot{\mathbf{u}}_k(\mathbf{s}_O)$, in other words, $\ddot{\mathbf{u}}_k(\mathbf{s}_O) = \Phi_O \Phi^{-1} \ddot{\mathbf{u}}_k(\mathbf{s})$. In the case that all DOF are observed, $\mathbf{s}_O = \mathbf{s}$, $\Phi_O \Phi^{-1} = I$, and the familiar 287 288 289 observation equation $\mathbf{y}_k = C\mathbf{x}_k$ is obtained.

In review of the standard model, the state variable represents all N DOF and the observations measure responses at N_O DOF. Thus, \mathbf{y}_k is an $N_O \times 1$ vector, \mathbf{x}_k is a $pN \times 1$ vector, the state matrix A is $pN \times pN$, the observation matrix C is $N_O \times pN$, and the mode shape regression matrix $\Phi_O \Phi^{-1}$ is $N_O \times N$ (recall M = N).

However, this model is unmanageably large when *N* is very large and thus is unsuitable for DSN. In this framework, the number of DOF (state variables) is coupled with the sensing nodes; they are 297 coincident. In other words, a high spatial resolution requires an 298 overly complex dynamic model. Furthermore, the state variable size 299 is very large (too large for system identification methods as one of 300 the users of such models). For common networks, the required com-301 putational efforts for model-order-selection-based structural modal 302 identification of fixed sensing networks are substantial (Chang and 303 Pakzad 2012; Pakzad and Fenves 2009) and greatly sensitive to the 304 size of the state variable (Matarazzo et al. 2015a). For a very large N, 305 this model is impractical for modal identification purposes. In con-306 sideration of an efficient model for DSN, it is illogical for all sensing 307 nodes and states to coincide as required by this model; it is thus de-308 sirable to implement a model capable of distinguishing between 309 these entities. 310

Modal State-Space Model

This subsection presents the second state-space model under 312 consideration for DSN: the modal state-space model. In this 313 approach, the observations represent the same entities as before, 314 i.e., $\mathbf{y}_k = \ddot{\mathbf{u}}_k(\mathbf{s}_O)$; however, the states, provided by Eq. (11), represent modal responses 316

$$_{k} \equiv \begin{bmatrix} \mathbf{q}_{k}^{\langle 1 \rangle} & \dots & \mathbf{q}_{k}^{\langle M \rangle} & \dot{\mathbf{q}}_{k}^{\langle 1 \rangle} & \dots & \dot{\mathbf{q}}_{k}^{\langle M \rangle} \end{bmatrix}^{T}$$
(11)

311

The sampled modal responses are defined for all time steps k = 3171, 2, ..., K and all modes m = 1, 2, ..., M, where $q_k^{\langle m \rangle}$ is the 318 sampled modal displacement for mode m at time step k; $\dot{q}_k^{\langle m \rangle}$ is 319 the sampled modal velocity for mode m at time step k; and $\ddot{q}_k^{\langle m \rangle}$ 320 is the sampled modal acceleration for mode m at time step k. 321

Along this new state variable, the remaining state terms are defined for modal space using M modal equations of motion (see Chapter 12 of Chopra 2007 for details). The modal mass matrix, modal stiffness matrix, modal damping matrix, and modal inputs in Eqs. (12) through (15) are found using modal superposition and the modal equations of motion 327

$$\bar{M} \equiv \Phi^T \bar{m} \Phi \tag{12}$$

$$\bar{K} \equiv \Phi^T \bar{k} \Phi \tag{13}$$

$$\bar{C} \equiv \Phi^T \bar{c} \Phi \tag{14}$$

$$\mathbf{v}_k \equiv \Phi^T \mathbf{\eta}_k \tag{15}$$

The full mode shape matrix and the submode shape matrix are 328 identical to those in the standard state-space model: $\Phi = \Phi^{\langle M \rangle}(\mathbf{s})$ is 329 an $N \times M$ matrix and $\Phi_O = \Phi^{\langle M \rangle}(\mathbf{s}_O)$ is an $N_O \times M$ matrix. The 330 modal state-space model parameters provided in Eqs. (16) through (20) are analogous to the physical model counterparts from Eqs. (4) 332 through (8) 333

$$A_c^{\langle M \rangle} \equiv \begin{bmatrix} 0 & I \\ -\bar{M}^{-1}\bar{K} & -\bar{M}^{-1}\bar{C} \end{bmatrix}$$
(16)

$$A^{\langle M \rangle} = \exp(A_c^{\langle M \rangle} \Delta t) \tag{17}$$

$$B_c^{\langle M \rangle} \equiv \begin{bmatrix} 0 \\ -\bar{M}^{-1}B_f \end{bmatrix}$$
(18)

$$B^{\langle M \rangle} = (A_c^{\langle M \rangle})^{-1} (A^{\langle M \rangle} - I) B_c^{\langle M \rangle}$$
(19)

395

396

397

$$C^{\langle M \rangle} \equiv C_a^{\langle M \rangle} [-\bar{M}^{-1}\bar{K} - \bar{M}^{-1}\bar{C}]$$
(20)

Finally, note $p_k^{\langle m \rangle}$ is the sampled modal input for mode *m* at time 334 335 step k

$$\mathbf{v}_k \equiv [\mathbf{p}_k^{\langle 1 \rangle} \quad \dots \quad \mathbf{p}_k^{\langle M \rangle}]^T \tag{21}$$

336 The *M* second-order differential equations representing the mo-337 dal equations of motion are expressed in first-order form through 338 the modal state Eq. (22) and a physical-modal observation Eq. (23) 339 for DSN

$$\mathbf{z}_{k} = A^{\langle M \rangle} \mathbf{z}_{k-1} + B^{\langle M \rangle} \mathbf{v}_{k-1}$$
(22)

$$\mathbf{y}_k = \Phi_O C^{\langle M \rangle} \mathbf{z}_k \tag{23}$$

340 The wording physical-modal is intended to acknowledge that observations and states are in different coordinate systems, physical 341 and modal, respectively. In this model, the observation matrix 342 343 represents modal coordinates; thus, mode shape regression, i.e., premultiplication by Φ^{-1} , is no longer necessary. In Eq. (23), modal 344 345 state responses are mapped to physical measurements through the modal observation matrix $C^{\langle M \rangle}$ and observation submode shape 346 matrix Φ_O . With this framework, the observation subvector \mathbf{s}_O and 347 348 corresponding submode shape matrix Φ_O account for variations in 349 sensor configurations; all other model parameters are preserved.

350 In review of the modal model, the state variable represents all M 351 modal responses and the observations measure physical responses 352 at N_O DOF. Thus, \mathbf{y}_k is an $N_O \times 1$ vector, \mathbf{z}_k is a $pM \times 1$ vector, the state matrix $A^{\langle M \rangle}$ is $pM \times pM$, the observation matrix $C^{\langle M \rangle}$ is 353 $M \times pM$, and the submode shape matrix for the observations 354 Φ_O is $N_O \times M$. Now if M = N, the model parameters are as large 355 356 as before, in which case, the modal model offers no evident im-357 provement over the standard model. However, the number of sens-358 ing nodes is no longer coupled with the state variable; thus, one issue with the previous model has been resolved. Furthermore, 359 if the modes are truncated so that M is considerably smaller than 360 N, for example, $M = N_0$ (note $N_0 \le M \le N$), then, the advantages 361 362 of this model become quite apparent. The number of modal responses included dictates the size of this modal model, whereas 363 364 the size of the standard model is defined by the total number of sensing nodes. Moreover, the practice of modal truncation, i.e., the 365 selection of M, is a familiar decision in structural dynamics for sys-366 367 tems with large DOF and the assumption leads clear theoretical 368 consequences [see section 19.7 in Chopra (2007)].

369 With the state variable decoupled from the model DOF, the state 370 size is independent of the total number of sensing nodes and the model complexity is reduced from pN in the standard model to 371 pM in this modal model. There are three main benefits of this re-372 duction: a significantly smaller model, model complexity is user 373 374 selected through M, and the significance of this selection is intuitive as it is equivalent to modal truncation. 375

376 In conclusion, the modal state-space model is an attractive 377 choice for modeling structural systems using DSN data. However, the model contains the following two pitfalls: 378

379 1. The states represent modal responses, while the observations 380 are in physical coordinates. In system identification, it is 381 counterintuitive to decompose the measured signal into modal 382 components without knowledge of the modal properties of the 383 structural system, i.e., prior to identification. Moreover, assuming $\mathbf{z}_k, A^{\langle M \rangle}$, and $C^{\langle M \rangle}$ are available, a coordinate transformation 384 would be required to extract corresponding spatial information 385 386 in the physical space. In short, with a modal state variable, physical mode shapes are not available directly after model 387 388 identification.

2. The submode shape matrix for the observations Φ_0 is a function 389 of the time step since the locations of the observations s_0 vary 390 over time, i.e., O = O(k). This feature yields a linear parameter varying (LPV) state-space model, complicating system identification procedures.

The following subsection presents a truncated physical model (TPM), which maintains the benefits of the modal state-space model and addresses the aforementioned challenges.

Truncated Physical Model

Previous state-space approaches in this paper have adapted existing 398 models to include DSN data as observations, primarily mapping 399 states to measured values using the submode shape for the obser-400 vations. This section presents a novel state-space technique for 401 DSN data: the truncated physical model (TPM). The TPM assumes 402 the modal state-space model (from the previous section) was the 403 result of a coordinate transformation T, which mapped modal states 404 **z** to truncated physical states \mathbf{x}^* via $\mathbf{x}^* = T\mathbf{z}$. Motivated from the 405 challenges of implementing the modal state-space model for DSN, 406 the goal of the TPM is to transform the modal matrices so that the 407 state variable represents responses in physical (not modal) coordi-408 nates. It will be shown that, after this transformation, a truncated 409 modal space yields a reduced (truncated) physical space for the 410 states while the sensing nodes are unaffected. The benefits of modal 411 truncation are mapped into a reduced, but not restricted, physical 412 state representation of the dynamic system. The assumed transfor-413 mation exclusively activates N_{α} user-selected DOF, specified by 414 $\mathbf{s}_{\alpha} \subset \mathbf{s}$ and $\mathbf{s}_{\alpha} \neq \mathbf{s}$ (otherwise, the benefits of this transformation 415 are lost), with $\Phi_{\alpha} = \Phi^{\langle M \rangle}(\mathbf{s}_{\alpha})$, where Φ_{α} is an $N_{\alpha} \times M$ matrix. 416 The locations specified by \mathbf{s}_{α} are hereafter named virtual probing 417 locations (VPL). It will be shown that the states are the responses at 418 these VPL. 419

For simplicity and minimum model size, it is assumed that the 420 number of observations in the DSN data matrix, the number of mo-421 dal responses included in the dynamic system, and the number of 422 VPL are all equal, i.e., $N_O = M = N_\alpha$; note the minimum value for 423 *M* is selected. The $pN_{\alpha} \times pM$ transformation matrix *T* is defined in 424 Eq. (24) with square, block diagonal entries Φ_{α} 425

$$T \equiv \begin{bmatrix} \Phi_{\alpha} & 0\\ 0 & \Phi_{\alpha} \end{bmatrix}$$
(24)

The transformation matrix relates TPM parameters (denoted 426 by superscript *) to modal model parameters [denoted by superscript $\langle M \rangle$] 428

$$A^{\langle M \rangle} \equiv T^{-1} A^* T \tag{25}$$

$$B^{\langle M \rangle} \equiv T^{-1} B^* \tag{26}$$

$$C^{\langle M \rangle} \equiv C^* T \tag{27}$$

The preceding equations are rearranged (back-transformed) to define TPM parameters in terms of modal model parameters

$$A^* = TA^{\langle M \rangle} T^{-1} \tag{28}$$

$$B^* = TB^{\langle M \rangle} \tag{29}$$

$$C^* = C^{\langle M \rangle} T^{-1} \tag{30}$$

The back-transformation results in physical responses at 431 \mathbf{s}_{α} , while the observations are unaltered. In other words, unlike 432

427

429

433 the modal model, the TPM is exclusively defined in physical 434 coordinates. Note $\mathbf{s}_{\alpha} \neq \mathbf{s}$ and \mathbf{s}_{α} contains significantly fewer 435 elements than \mathbf{s} , i.e., $N_{\alpha} \ll N$; otherwise, the benefits of this 436 back-transformation would be lost. The $pN_{\alpha} \times 1$ TPM state vector 437 is defined by the spatial vector \mathbf{s}_{α} ; therefore, the responses at the 438 VPL dictate the dynamic model

$$\mathbf{x}_{k}^{*} \equiv \begin{bmatrix} \mathbf{u}_{k}(\mathbf{s}_{\alpha}) \\ \dot{\mathbf{u}}_{k}(\mathbf{s}_{\alpha}) \end{bmatrix}$$
(31)

439 It is important to reiterate that this reduction in physical space 440 is not restrictive. In other words, the VPL can represent any user-441 selected DOF subset defined by $\mathbf{s}_{\alpha} \subset \mathbf{s}$ and *T* accordingly, a trait 442 unique to the TPM

$$\mathbf{x}_k^* = A^* \mathbf{x}_{k-1}^* + B^* \mathbf{v}_{k-1} \tag{32}$$

$$\mathbf{y}_k = \Omega \Phi_\alpha C^* \mathbf{x}_k^* \tag{33}$$

443 In Eq. (34), an MSR term is represented by Ω , an $N_O \times M$ 444 matrix

$$\Omega \equiv \Phi_O \Phi_O^{-1} \tag{34}$$

445 Similar to the MSR term found in Eq. (11) for the standard statespace model, the term in Eq. (34) represents the regression of the 446 447 ordinates at the VPL responses (defined by \mathbf{s}_{α}) on to those at the observations (defined by \mathbf{s}_O). In other words, $\Omega = \Phi_O \Phi_\alpha^{-1}$ maps 448 449 the VPL responses $\ddot{\mathbf{u}}_k(\mathbf{s}_{\alpha}) = \Phi_{\alpha} C^* \mathbf{x}_k^*$ to the observations $\ddot{\mathbf{u}}_k(\mathbf{s}_O)$, i.e., $\ddot{\mathbf{u}}_k(\mathbf{s}_{\alpha}) = \Psi \ddot{\mathbf{u}}_k(\mathbf{s}_{\alpha})$. Unlike the modal model, the TPM has a 450 451 physical state variable, so that when the state matrix and the ob-452 servation matrix are available, the corresponding mode shapes 453 cover VPL nodes. For example, in system identification, physical 454 mode shapes can be computed directly from these model param-455 eters: the eigendecomposition of A^* yields natural frequencies 456 and damping ratios, and $\Phi_{\alpha}C^*$ provides submode shapes at VPL. 457 In review of the TPM, the size and locations (VPL) of the 458 truncated physical states are user defined, through N_{α} and \mathbf{s}_{α} , 459 respectively. Also, truncated physical states are exact truncated 460 physical responses at the VPL. In general, the observation vector 461 \mathbf{y}_k is $N_O \times 1$, the truncated physical state vector \mathbf{x}_k^* is $pN_\alpha \times 1$, the 462 truncated physical state matrix A^* is $pN_{\alpha} \times pN_{\alpha}$, the truncated 463 physical observation matrix C^* is $M \times pN_{\alpha}$, the submode shape 464 term for VPL Φ_{α} is $N_{\alpha} \times M$, and the MSR term Ω is $N_{O} \times M$. With 465 the assumption for minimum model size $N_{\alpha} = N_{O} = M$, the model complexity is reduced significantly and becomes directly related to 466 the number of observations in the DSN data matrix. More specifi-467 cally, \mathbf{x}_k^* is $pN_O \times 1$, A^* is $pN_O \times pN_O$, C^* is $N_O \times pN_O$, Φ_α is 468 469 $N_O \times N_O$, and Ω is $N_O \times N_O$.

In conclusion, the TPM establishes an intuitive relationship
between the observation size of the DSN data matrix and the
complexity of the underlying dynamic states. More importantly,

model complexity and state DOF are independent of the full set of sensing nodes. This is a vast improvement on the coupled nature between states and sensing nodes observed in the standard statespace model. Additionally, with physical, user-defined VPL states, the interpretation of identified modal properties is simplified.

473

474

475

476

477

478

479

480

481

482

483

484

485

486

495

As with other state-space models (Matarazzo and Pakzad 2015a; Matarazzo et al. 2015a; Peeters and De Roeck 1999), an additive noise term can be included in the observation equation of the TPM. Eq. (33) can be modified to $\mathbf{y}_k = \Omega \Phi_\alpha C^* \mathbf{x}_k^* + \mathbf{w}_k$, to properly, and simply, include sensor noise \mathbf{w}_k , which is independent of the true structural response. There are no particular restrictions on sampling frequency required by the TPM or DSN data other than consideration of the Nyquist frequency (Oppenheim et al. 1999) to prevent temporal aliasing.

It is important to mention that Ω is a time-variant parameter 487 because it is a function of Φ_{Ω} . The merit of Ω is that it can be ap-488 proximated efficiently by a basis function for spatial reconstruction, 489 without knowledge of the true structural mode shapes. With this 490 approximation, the challenges of system identification of a LPV 491 state-space model (as mentioned at the end of the previous section) 492 can be eliminated. This topic is further discussed in the following 493 section. 494

Mode Shape Regression Using Basis Functions

In this section, the role of basis functions for the use in the TPM 496 is discussed. It is shown that the MSR term Ω [introduced in 497 Eq. (34)] can be approximated by the use of basis functions. Fur-498 thermore, accurate estimates of DSN data in time and frequency 499 domain become available in the TPM through a simple technique, 500 without additional use of true structural mode shapes. Moheimani 501 et al. (2003) presented linear reconstruction of structural mode 502 shapes using Shannon sampling theorem for discrete signals, 503 henceforth WKS (Whitaker, Kotelnikov, Shannon) theory. Portions 504 of this theorem and its extensions can be attributed to Whittaker 505 (1915, 1928), Kotelnikov (1933), or Shannon (1998); in this paper, 506 the term WKS is adopted from Jerri (1977), which refers to the 507 authors' collective contributions. The application of WKS is revis-508 ited using nomenclature familiar to the previous section; then, the 509 relation is adapted for use in the truncated physical state-510 space model. 511

The approach begins with an ideal, regular sampling case: first 512 assume sensing nodes are defined by $\mathbf{s} = [s_1 \ s_2 \ \dots \ s_N]^T$. 513 Eq. (35) below reformulates Eq. 7.39 from Moheimani et al. 514 (2003) for approximation of the $N \times M$ full mode shape $\Phi = 515 \Phi^{\langle M \rangle}(\mathbf{s})$ using an $N_{\chi} \times M$ subset mode shape $\Phi_{\chi} = \Phi^{\langle M \rangle}(\mathbf{s}_{\chi})$, 516 where $\mathbf{s}_{\chi} = [s_{\chi_1} \ s_{\chi_2} \ \dots \ s_{\chi_{\beta}}]^T$ is uniformly spaced on the 517 structure at Δs_{χ} and $\mathbf{s}_{\chi} \subset \mathbf{s}$ 518

$$\hat{\Phi} = \left[\operatorname{sinc}\left(\frac{1}{\Delta s_{\chi}}(\mathbf{s} - s_{\chi_{1}})\right) \quad \operatorname{sinc}\left(\frac{1}{\Delta s_{\chi}}(\mathbf{s} - s_{\chi_{2}})\right) \quad \dots \quad \operatorname{sinc}\left(\frac{1}{\Delta s_{\chi}}(\mathbf{s} - s_{N_{\chi}})\right)\right]\Phi_{\chi}$$
(35)

519 Note the estimate $\hat{\Phi}$ is an $N \times M$ matrix, which is exact at N_{χ} 520 subset locations, i.e., $\hat{\Phi}(\mathbf{s}_{\chi}) = \Phi(\mathbf{s}_{\chi}) = \Phi_{\chi}$, because $\operatorname{sinc}(0) = 1$. 521 Mode shape ordinate approximations at remaining $N - N_{\chi}$ loca-522 tions, i.e., $\hat{\Phi}(\mathbf{s} \not\subset \mathbf{s}_{\chi})$, are interpolated through this reconstruction. 523 As in the case with temporal sampling, it is essential to anticipate 524 the highest expected frequency content of the wave when selecting a spatial sampling frequency. In the case of a simply supported beam with *N* uniformly spaced nodes and length *L*, in order to avoid spatial aliasing, sensing nodes must be spaced so that $\Delta s_{\chi} < 527$ *L/N*. Additional details on the reconstruction accuracy and its corresponding error at sensing nodes, namely $\varepsilon \equiv ||\hat{\Phi} - \Phi||_2$, 529 can be found in reconstruction literature (Moheimani et al. 2003; 530 531 Jagerman and Fogel 1956; Jerri 1977; Stenger 1976; Whittaker 532 1915).

The next step will focus on the interpolated portions of the mode shape estimates. Two subsets of sensing nodes and their corresponding submode shapes will be defined, and then related using the same aforementioned theory. Finally, the relation between the submode shapes will be rearranged, to prove that the *sinc* basis function approximates the MSR term in the TPM.

539 Consider WKS for the problem of estimating modal ordinates 540 at a different subset mode shape matrix, say Φ_{δ} , using Φ_{γ} . In other

words, define a different subset of modal ordinates $\Phi_{\delta} = \Phi^{\langle M \rangle}(\mathbf{s}_{\delta})$ 541 of equal size, i.e., $N_{\delta} = N_{\chi}$, where $\mathbf{s}_{\delta} = \begin{bmatrix} s_{\delta_1} & s_{\delta_2} & \dots & s_{N_{\delta}} \end{bmatrix}$ 542 and $\mathbf{s}_{\delta} \subset \mathbf{s}$. Additionally, assume no overlapping locations between 543 these two sensing node subsets, i.e., their union is null $\mathbf{s}_{\gamma} \cap \mathbf{s}_{\delta} = \emptyset$ 544 (this is to demonstrate maximum utility; it is not a requirement). The 545 estimation of Φ_{δ} is given in Eq. (36), where the WKS reconstruc-546 tion has been adjusted to exclusively represent interpolation. 547 More specifically, the following equation defines linear regression 548 of one set of modal ordinates onto another where the entries of 549 the basis function matrix $\Omega_{\rm sinc}$ are the regression coefficients 550

$$\hat{\Phi}_{\delta} = \left[\operatorname{sinc} \left(\frac{1}{\Delta s_{\chi}} (\mathbf{s}_{\delta} - s_{\chi_{1}}) \right) \quad \operatorname{sinc} \left(\frac{1}{\Delta s_{\chi}} (\mathbf{s}_{\delta} - s_{\chi_{2}}) \right) \quad \dots \quad \operatorname{sinc} \left(\frac{1}{\Delta s_{\chi}} (\mathbf{s}_{\delta} - s_{N_{\chi}}) \right) \right] \Phi_{\chi}$$
$$\hat{\Phi}_{\delta} = \Omega_{\operatorname{sinc}} \Phi_{\chi} \tag{36}$$

551 Both sides of the equation above are postmultiplied by Φ_{χ}^{-1} 552 resulting in Eq. (37)

$$\Omega_{\rm sinc} = \hat{\Phi}_{\delta} \Phi_{\chi}^{-1} \tag{37}$$

553 Eq. (37) has a great significance in the context of the TPM. 554 The left-hand side of Eq. (37) is the sinc basis evaluated at the lags between locations \mathbf{s}_{δ} and \mathbf{s}_{χ} ; the right-hand side of Eq. (37) 555 is an approximation for the MSR term that relates $\ddot{\mathbf{u}}_k(\mathbf{s}_{\delta})$ 556 557 and $\ddot{\mathbf{u}}_k(\mathbf{s}_{\gamma})$ to each other. In other words, the *sinc* basis function 558 approximates the MSR term. Furthermore, with $\Phi_{\delta} = \Phi_{O}$ and $\Phi_{\chi} = \Phi_{\alpha}$, through the proper selections of \mathbf{s}_{δ} and \mathbf{s}_{χ} , the *sinc* basis 559 560 matrix is an estimator for Ω [found in the TPM, Eq. (34)]. Sinc is not the only basis capable of estimating the MSR term. 561 As discussed in Butzer et al. (1986), Moheimani et al. (2003), 562 563 and Unser (1999), B-splines are a computationally efficient replacement for a sinc basis and carry useful curvature and deriva-564 565 tive properties. The substitution in Eq. (38) provides an estimate for Ω , which is, in general, less accurate than Ω_{sinc} ; however, 566 B-splines provide the "best performance for the least complexity" 567 (Unser 1999) 568

$$\Omega_{\text{spline}} = \begin{bmatrix} \beta^n (\mathbf{s}_{\delta} - s_{\chi_1}) & \beta^n (\mathbf{s}_{\delta} - s_{\chi_2}) & \dots & \beta^n (\mathbf{s}_{\delta} - s_{N_{\chi}}) \end{bmatrix}$$
(38)

The variable complexity and performance of *B-splines* is characterized by the selection of degree *n*. In many applications, the cubic spline, n = 3, is a popular choice due to its minimum curvature property, and in fact, as the *spline* degree goes to infinity, the cardinal *spline* filter approaches the ideal *sinc* filter (Aldroubi et al. 1992).

575 Most importantly, the requirement of uniformly spaced sensing 576 nodes is not necessarily a restriction since in the TPM, the VPL are arbitrary; they are chosen by the user out of all sensing nodes. 577 578 Therefore, the user can simply program VPL to be uniformly 579 spaced nodes and achieve optimal results (maximum accuracy of 580 the MSR approximation). Also note, for irregular or nonperiodic 581 VPL, the WKS relations presented in this section remain appli-582 cable; however, the corresponding error has a different form 583 (Beutler 1961, 1966). It is recommended that the VPL are selected 584 to be uniformly spaced.

Processing Data from Novel Sensing Techniques

This section presents novel sensing applications of the modal model and the truncated physical model (TPM). As discussed in previously, the TPM computes dynamic sensor network (DSN) data efficiently at a model complexity, which depends on the modal truncation—not the quantity of sensing nodes. The implementation of a minimum complexity TPM to compute DSN data is essential to the eventual practice of such sensing systems. The following applications have three primary objectives:

- 1. Demonstrate that the proposed TPM provides responses identical to the theory. In these studies, the theoretical solution is represented by the modal state-space model, which provides the exact structural responses truncated to *M* modes.
- 2. Quantify the accuracy of the mode shape regression (MSR) term approximations computed using *sinc* and *spline* basis functions.
- 3. Provide two examples of novel sensing techniques that can be modeled exactly and efficiently (at the minimum model size) using DSN data and the TPM.

In both applications, the SHM of a flexible beam structure using 603 5,000 sensing nodes is considered. The high-resolution mobile 604 sensing case exemplifies online DSN data, while the BIGDATA 605 case demonstrates offline DSN data. In each case, the spatial 606 discontinuities in the DSN data matrix have a different source. 607 In high-resolution sensing, the discontinuities are due to the physi-608 cal movement of the sensors, while in BIGDATA, they are a result 609 of user-selected sensor scheduling, after data collection. Moreover, 610 the specific offline DSN application extracted from the raw data 611 represents only a single hypothetical data set out of the voluminous 612 possibilities available with BIGDATA. 613

High-Resolution Mobile Sensing Application

In this section, the response of a flexible simple beam is measured615by 19 mobile sensors, which scan 5,000 sensing nodes. Four mod-616els are considered to simulate the resulting online DSN data set:617modal model, TPM, TPM with *sinc* bases, and TPM with cubic618*splines*. The modal and TPM are exact and theoretically equivalent,619while the TPM with a basis function is approximate.620

In this application, a 5,000-DOF beam is subjected to a vertical white noise ground motion at the supports with a frequency cut off at 30 Hz. The natural vibration properties of the beam are provided in Table 1, with natural frequencies ranging from 0.27 to 98.19 Hz. 624

585

586

587

588

589

590

591

592

593

594

595

596

597

598

599

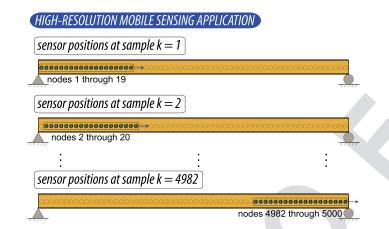
600

601

602

Table 1. First 19 Natural Vibration Properties of 5,000-DOF Beam

	1 /				
T1:1	Mode	Frequency (Hz)	Damping (%)		
T1:2	1	0.273	0.027		
T1:3	2	1.09	0.108		
T1:4	3	2.45	0.244		
T1:5	4	4.35	0.434		
T1:6	5	6.80	0.678		
T1:7	6	9.79	0.977		
T1:8	7	13.33	1.33		
T1:9	8	17.41	1.74		
T1:10	9	22.03	2.20		
T1:11	10	27.20	2.71		
T1:12	11	32.91	3.28		
T1:13	12	39.17	3.91		
T1:14	13	45.97	4.58		
T1:15	14	53.31	5.32		
T1:16	15	61.20	6.10		
T1:17	16	69.63	6.94		
T1:18	17	78.60	7.84		
T1:19	18	88.12	8.79		
T1:20	19	98.19	9.79		
	-				



F3:1 Fig. 3. Positions of mobile sensors at selected samples in high-resolution mobile sensing application; 19 sensors scan 5,000 sensing nodes as a group, shifting rightward to the next node after each sample

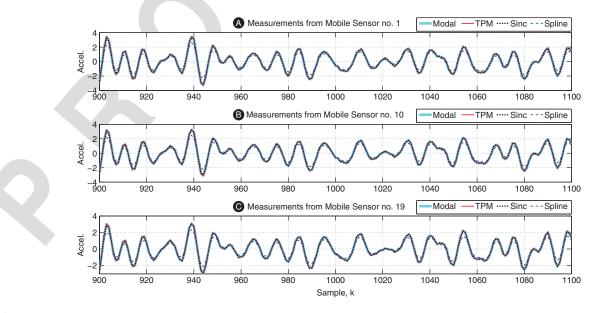
Fig. 3 depicts the mobile sensing network for this application, 625 which samples at a rate of 200 Hz. The DSN is a group of 19 neigh-626 boring sensors that scan the structural response by shifting together, 627 after each sample, in increments of one sensing node. At sample 628 k = 1, the sensor group measures responses at sensing nodes 1 629 through 19; at sample k = 2, they observe sensing nodes 2 through 630 20; finally, at sample k = K = 4,982, they observe sensing nodes 631 4,982 through 5,000. The resulting online DSN data matrix is 632 $19 \times 4,982$ and includes information from all 5,000 sensing nodes. 633

The exact modal responses are calculated for the first 19 modes 634 (M = 19) using a modal state-space model. With this information, 635 the exact truncated responses can be computed at all DOF through 636 the use of a spatially dense mode shape vector. However, it is only 637 necessary to compute responses at locations and times where the 638 DSN is scheduled to cover. As presented earlier, the submode shape 639 term is added to the modal state-space model to calculate the ob-640 servations of an online DSN. The resulting DSN data are the exact 641 truncated measurements. 642

Using the same loading, the TPM is constructed in accordance 643 with its introductory section with 19 VPL selected uniformly across 644 the beam; thus, for the minimum model size, 19 structural modes 645 were included. The modal model computed DSN data directly from 646 modal responses of the state variable. The TPM computes DSN 647 data (observations) from the truncated physical states, the exact 648 truncated physical responses at VPL DOF. 649

Mobile Sensing Results

In Fig. 4, the responses at mobile sensors 1, 10, and 19 are com-651 pared over a selected range of samples. The individual responses of 652 the moving sensors are redundant as the range of sensing nodes 653 covered by the group is quite small, covering only 0.38% of the 654 beam at each sample. The DSN data from the modal and TPM 655 are nearly identical, as they are theoretically equivalent; any differ-656 ences are the result of computational error, predominantly, the ma-657 trix inversion required in the TPM by $\Omega = \Phi_0 \Phi_\alpha^{-1}$. As expected, 658 the computational error for the mobile sensing DSN data (over 659 94,000 entries) is small, with a mean squared error (MSE) equal 660 to 16.54×10^{-5} . 661



F4:1 **Fig. 4.** Comparison of data from modal, TPM, and TPM with basis approximations for samples 900 through 1,100 for mobile: (a) sensor 1; (b) sensor F4:2 10; (c) sensor 19

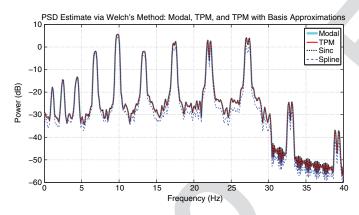
Table 2. Comparison of Online DSN Data in Mobile Sensing Application

T2:1	Error type	Sum of squares in time domain	Time domain MSE	Sum of squares of PSD	PSD MSE
T2:2	Computational $\sum_{k,s_o} (Y_{\text{modal}} - Y_{\text{TPM}})^2$	15.66	16.54×10^{-5}	11.72×10^{-4}	60.21×10^{-9}
T2:3	Sinc basis $\sum_{k,s_o} (Y_{\text{TPM}} - \hat{Y}_{\text{TPM}}^{\text{sinc}})^2$	1,779.79	18.80×10^{-3}	50.00×10^{-3}	25.67×10^{-7}
T2:4	Cubic <i>B-spline</i> $\sum_{k,s_o} (Y_{\text{TPM}} - \hat{Y}_{\text{TPM}}^{\text{spline}})^2$	38,958.51	41.16×10^{-2}	351.92	18.07×10^{-3}

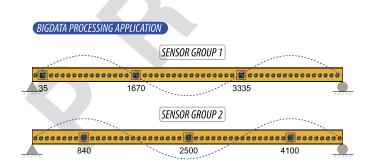
Note: Sum of squared errors and mean squared errors (MSE) are computed among the four DSN data sets: modal, TPM, TPM with *sinc*, and TPM with *spline*. Time domain errors are computed directly from DSN data matrices while power spectral density (PSD) errors are computed from PSD estimates using Welch's method. DSN data matrices are $19 \times 4,982$.

Fig. 4 also compares measurements from mobile sensors 1, 10, and 19, computed by the TPM with *sinc* and cubic *spline* bases. The overall behavior of the mobile sensor data is captured well by both approximations. As quantified in Table 2, the *sinc* function outperforms the cubic spline in accuracy by an order of magnitude, with MSE equal to 18.80×10^{-3} versus 41.16×10^{-2} .

Fig. 5 displays the power spectral density (PSD) estimate, com-668 puted using the average of Welch's method over all 19 sensors. The 669 data from the modal model and TPM contain nearly identical PSD 670 estimates with MSE equal to 60.21×10^{-9} . Fig. 5 also provides the 671 PSD estimate for the TPM data with those obtained using sinc and 672 673 cubic spline basis approximations. The sinc PSD is coincident with 674 previous TPM PSD, while the spline has, overall, less power. In 675 Table 2, the approximation error is detailed, in which cubic *spline* MSE is four orders of magnitude higher than the MSE from sinc. 676



F5:1 Fig. 5. Averaged PSD estimates of data from modal, TPM, and TPMF5:2 with basis approximations computed via Welch's method for high-F5:3 resolution mobile sensing application



F6:1 Fig. 6. BIGDATA processing application considers the switching between two groups; group 1 consists of sensing nodes 35, 1,670, and
3,335 while group 2 covers nodes 840, 2,500, and 4,100; the thirdmode shape of the structure is superimposed to demonstrate the expected node responses to a third-mode harmonic excitation

BIGDATA Processing Application

In this subsection, the response of the simple beam, with modal properties given in Table 1, is measured using 5,000 sensors, one fixed at each sensing node. A harmonic load with frequency 2.45 Hz is applied at sensing node 2,500, resulting in an ideal forced, third-mode structural response. With responses available at 5,000 locations, the processing options are overwhelming. In this application, only three observations are considered in the offline DSN data set. Therefore, for the minimum model order, three modes are included in the TPM.

677

678

679

680

681

682

683

684

685

686

687

688

689

690

691

692

693

694

695

696

697

698

699

700

701

702

703

704

714

715

716

717

718

719

720

721

722

723

724

725

726

727

As pictured in Fig. 6, these observations are programmed to represent measurements in two specific sensing groups. Group 1 includes sensing nodes 35, 1,670, and 3,335, and group 2 includes sensing nodes 840, 2,500, and 4,100. The offline DSN matrix consists of data from group 1 until the 500th sample, when the observations switch to group 2. Clearly, this selection only represents one possible subset out of the many possibilities given in the BIGDATA population. Moreover, as pictured in Fig. 6, responses in group 1 are expected to be very small in magnitude due to the proximity of the nodes to zero-valued third-mode ordinates, and thus contain little information. When group 2 is selected, the responses are expected to have large values as the sensing nodes coincide with maximal third-mode ordinates.

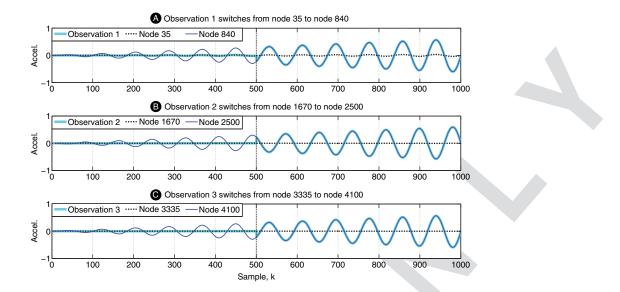
As in the high-resolution mobile sensing application, the modal model, TPM, TPM with *sinc* bases, and TPM with cubic *splines* are considered to simulate the offline DSN data set. The DSN data are compared in time and frequency domain.

BIGDATA Results

In Fig. 7, a plot of each observation is provided along relevant 705 node responses to display the observation switching scheme. For 706 example, in Fig. 7(a), observation 1 is shown with nodes 35 and 707 840 for all samples. During samples 1 through 499, sensing group 708 1 is active, so that observation 1 represents samples at node 35. 709 During samples 500 through 1,000, sensing group 2 is active and 710 observation 1 represents samples at node 840. Figs. 7(b and c) 711 show a parallel relationship with observation 2 and nodes 1,670 and 712 2,500, as well as observation 3 and nodes 3,335 and 4,100. 713

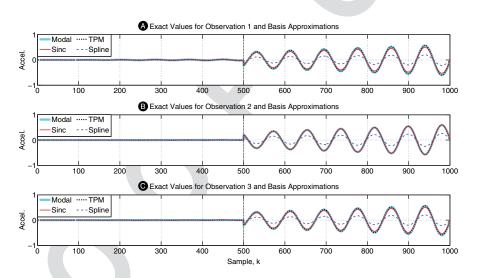
Fig. 8 provides the true DSN values computed using the modal and TPM. TPM approximations with *sinc* and cubic *spline* basis are also included. As mentioned previously, the modal and TPM data sets are theoretically exact, so the only differences are computational as indicated with an MSE equal to 46.54×10^{-6} in Table 3. The *sinc* and *B-spline* approximations capture the overall behavior; however, the superior accuracy of sinc basis is evident. Quantitatively, the MSE for the *sinc* approximation is two orders of magnitude lower than that of the cubic *B-spline*.

In Fig. 9, the PSD estimates are plotted for all four models. Consistent with previous analyses, the overall behavior of the response is captured by all four models. The modal model, TPM, and TPM with *sinc* basis approximation are all in agreement, while there is overall considerably less power in the *B-spline* approximation.



F7:1 **Fig. 7.** Each observation of the offline DSN data matrix is plotted with relevant sensor node responses as scheduled in the BIGDATA application; F7:2 group switching occurs at sample 500: (a) observation 1 with responses at nodes 35 and 840; as scheduled in the BIGDATA application, observation 1 F7:3 switches from sensing node 35 to 840; (b) observation 2 with responses at nodes 1,670 and 2,500; (c) observation 3 with responses at nodes 3,335





F8:1 Fig. 8. (a) Modeled BIGDATA offline DSN observation 1 computed using the modal model, TPM, and TPM with basis approximations; (b) the
 modeled BIGDATA offline DSN observation 2 computed using the modal model, TPM, and TPM with basis approximations; (c) modeled BIGDATA
 F8:3 offline DSN observation 3 computed using the modal model, TPM, and TPM with basis approximations;

T3:1	Error type	Sum of squares in time domain	Time domain MSE	Sum of squares of PSD	PSD MSE
T3:2	Computational $\sum_{k,s_o} (Y_{\text{modal}} - Y_{\text{TPM}})^2$	13.96×10^{-2}	46.54×10^{-6}	11.17×10^{-9}	28.87×10^{-12}
T3:3	Sinc basis $\sum_{k,s_o} (Y_{\text{TPM}} - \hat{Y}_{\text{TPM}}^{\text{sinc}})^2$	1.61	$53.81 imes 10^{-5}$	16.82×10^{-5}	43.46×10^{-8}
T3:4	Cubic <i>B-spline</i> $\sum_{k,s_o} (Y_{\text{TPM}} - \hat{Y}_{\text{TPM}}^{\text{spline}})^2$	61.50	20.50×10^{-3}	38.84×10^{-4}	10.04×10^{-6}

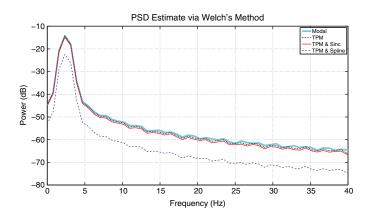
Note: Sum of squared errors and mean squared errors (MSE) are computed among the four DSN data sets: modal, TPM, TPM with *sinc*, and TPM with *spline*. Time domain errors are computed directly from DSN data matrices while power spectral density (PSD) errors are computed from PSD estimates using Welch's method. DSN data matrices are $3 \times 1,000$.

728 However, in this case, the computational PSD MSE is 28.87×10^{-12} and the *sinc* approximation PSD MSE is 43.46×10^{-8} ,

729 10 and the sine approximation 13D MSE is $43.40 \times 10^{\circ}$, 730 two orders of magnitude lower than the *B-spline* PSD MSE.

730 The accuracy of the *spline* MSR approximation can be improved

by including additional modes in the model. In the previous appli-
cation where 19 modes were considered, there was little discrep-
ancy between the TPM with a *spline* basis and the TPM with a *sinc*732733
basis. Thus, when the number of modes included in the TPM is735



F9:1 Fig. 9. Averaged PSD estimates of TPM data and basis approx-F9:2 imations, computed via Welch's method for BIGDATA processing F9:3 application

736 small or moderate, the *sinc* basis is recommended to approximate 737 the MSR term.

While this application utilized many sensors to capture a simple 738 739 response, it is clear that observation switching through offline DSN 740 provides a powerful technique for generating aggregate data sets 741 with dense structural information. For example, an optimal sensor 742 network strategy (Chang and Pakzad 2014; Guo et al. 2004; 743 Papadimitriou 2004) could be implemented to extract an optimal 744 (by some measure) offline DSN data matrix from an available fixed 745 sensor network, perhaps, a BIGDATA population. Moreover, in 746 general, the goal is to use this strategy to build smart DSN data 747 sets, which carry rich structural information in a compact size.

748 Conclusions

749 In this paper, dynamic sensor network (DSN) data sets were 750 proposed to efficiently store measurements from a very large 751 quantity of sensing nodes in a relatively small matrix. Note that 752 a physical DSN system is not required to obtain a DSN data ma-753 trix. Spatial discontinuities in DSN data matrices enable a high 754 capacity for storing spatial information. In the section "Dynamic 755 Sensor Network Data" the concept of DSN was formally intro-756 duced and the roles of sensors, sensing nodes, and observations 757 were defined. General types of DSN, such as online and offline, 758 were also established.

In the section "Exact State-Space Models for Dynamic Sensor 759 Networks" classical state-space models were modified to represent 760 DSN data sets and associated modeling challenges were identified. 761 Primarily, in the standard state-space model, the state variable 762 763 coincides with sensing nodes; thus, a dense spatial grid dictates an overly complex dynamic model. The truncated physical model 764 765 (TPM) was proposed as a computationally efficient technique to address these challenges. The TPM is theoretically equivalent to 766 767 a modal state-space model with DSN observations (presented ear-768 lier in this paper) and establishes an intuitive relationship between 769 the observation size of the DSN data matrix and the complexity of 770 the underlying dynamic states. Additional benefits of the TPM in-771 clude an unrestricted physical state variable, which represents user-772 defined virtual probing locations (VPL); in other words, the user 773 may choose which sensing nodes define the state variable. This is a 774 vast improvement on the coupled nature between states and sensing 775 nodes seen in the standard state-space model.

776 In the section "Mode Shape Regression Using Basis Functions" 777 the approximation of the mode shape regression term, defined in

Eq. (34) of the TPM, through basis functions is discussed. Using 778 the Whitaker-Kotelnikov-Shannon (WKS) reconstruction theory, 779 sinc or spline bases are implemented in the TPM to bypass addi-780 tional mode shape matrices in the observation equation. The result 781 simplifies the subsequent system identification of TPM by elimi-782 nating the complex linear parameter varying (LPV) nature of the 783 model, thus avoiding LPV-type identification algorithms. 784

High-resolution mobile sensing and BIGDATA processing 785 applications were considered in the section "Processing Data from 786 Novel Sensing Techniques" to exemplify novel sensing explora-787 tions with DSN data. In high-resolution mobile sensing, informa-788 tion from the responses at 5,000 sensing nodes was measured by 19 789 moving sensors and condensed into a 19×1 vector at each sample, 790 and modeled with a 38×1 state variable. Note theoretically, in the 791 standard state space model, the state variable would be restricted to 792 a $10,000 \times 1$ vector. The BIGDATA processing application demon-793 strated the versatility in offline DSN data sets and the ability to 794 process a smart subset. Given a very dense fixed sensor array 795 and an enormous data matrix, offline DSN provide the ability to 796 build an information-packed data matrix from user-selected sensor 797 measurements. In the application, the second sensor group 798 contained significantly more structural information than the first 799 sensor group, exhibiting the utility in offline DSN for processing 800 BIGDATA. 801

Acknowledgments

Research funding is partially provided by the National Science 803 Foundation through Grant No. CMMI-1351537 by Hazard Mitiga-804 tion and Structural Engineering program, and by a grant from the 805 Commonwealth of Pennsylvania, Department of Community and 806 Economic Development, through the Pennsylvania Infrastructure 807 Technology Alliance (PITA). 808

Notation

The following symbols are used in this paper:	810
$A =$ standard state matrix; size is $pN \times pN$;	812
A_c = continuous-time state matrix of the standard	813
state-space model; size is $pN \times pN$;	815
$A^{\langle M \rangle}$ = modal state matrix; size is $pM \times pM$ and $M \ll N$;	810
$A_c^{\langle M \rangle}$ = continuous-time state matrix of the modal state-space	819
model; size is $pM \times pM$ and $M \ll N$;	820
A^* = TPM state matrix; size is $pN_{\alpha} \times pN_{\alpha}$ and	822
$N_{\alpha} = N_O = M;$	823
B = standard state input matrix; size is $pN \times N$;	824
B_c = continuous-time state matrix of the standard	820
state-space model; size is $pN \times N$;	828
$B^{\langle M \rangle}_{\langle M \rangle}$ = modal state input matrix; size is $pM \times M$ and $M \ll N$;	839
$B_c^{\langle M \rangle}$ = continuous-time state matrix of the modal state-space	832
model; size is $pM \times M$ and $M \ll N$;	833
B^* = TPM state input matrix; size is $pN_{\alpha} \times N_{\alpha}$ and	834
$N_{lpha} = N_O = M;$	836
B_f = scaling matrix for the applied forces in the	838
continuous-time equation of motion; size is $N \times N$	839
\overline{C} = modal damping matrix for M modes; size is $M \times M$	840
and $M \ll N$;	842
C = standard observation matrix; size is $N_O \times pN$;	843

- standard observation matrix; size is $N_0 \times pN_1$
- C_a = measurement conversion matrix, as described in section 7.2.1 of Juang and Phan 2001; size is $N_O \times N_O$
- $C^{\langle M \rangle}$ = modal observation matrix; size is $M \times pM$ and $M \ll N;$

11

802

809

846

847

848

849

 $C_a^{\langle M \rangle}$ = modal measurement conversion matrix; size is 853 854 $M \times M;$ C^* = TPM observation matrix; size is $M \times pN_{\alpha}$ and 856 857 $N_{\alpha} = N_{O} = M;$ \bar{c} = structural damping matrix for N DOF; size is $N \times N$; 859 K = number of time samples (number of rows in DSN data 860 matrix): scalar: 862 \overline{K} = modal stiffness matrix for M modes; size is $M \times M$ 863 865 and $M \ll N$; \bar{k} = structural stiffness matrix for N DOF; size is $N \times N$; 866 869 k = time step index: 870 M = number of modes included in analysis; scalar; \overline{M} = modal mass matrix for M modes; size is $M \times M$ and 873 874 $M \ll N$: \bar{m} = structural mass matrix for N DOF; size is $N \times N$; 876 878 m = mode index: N = total number of sensing nodes in model (DOF); scalar;880 N_{O} = observation size (number of columns in DSN data 882 883 matrix); scalar; N_{mc} = number of sensors (measurement channels) in data set; 884 886 scalar; 888 $n = \text{degree of } B\text{-spline } \Omega_{\text{spline}}; \text{ scalar};$ 899 N_{χ} = total number of sensing nodes in subset χ ; 891 N_{α} = total number of VPL, i.e., sensing nodes in subset α ; 893 N_{δ} = total number of sensing nodes in subset δ ; 896 O = spatial subset corresponding to observations; when the 897 observations are DSN data, this is a time-variant 898 function, i.e., O = O(k); 900 p = state-space model order (theoretically, p = 2); scalar; $p_{k}^{\langle m \rangle}$ = modal input for mode *m* at time step *k*; 902 $q_k^{\langle m \rangle}$ = modal displacement for mode *m* at time step *k*; 903 $\dot{q}_{k}^{\langle m \rangle}$ = modal velocity for mode *m* at time step *k*; 906 $\ddot{q}_{k}^{\langle m \rangle}$ = modal acceleration for mode *m* at time step *k*; 908 S_{Q} = sensor-position matrix for observations; size is 900 911 $K \times N_O$ \mathbf{s} = vector describing locations of all sensing nodes; size is 913 914 $N \times 1;$ $\Delta \mathbf{s}_i$ = spacing for a uniform sensing node subset \mathbf{s}_i ; 916 918 \mathbf{s}_i = vector describing locations of sensing nodes in subset 919 *i*; size is $N_i \times 1$; T =coordinate transformation from modal to truncated 920 physical coordinates; size is $pN_{\alpha} \times pM$ and 922 923 $N_{\alpha} = N_O = M;$ 924 $\mathbf{u}_k(\mathbf{s}_i)$ = vector of displacement responses at \mathbf{s}_i and time step k; 926 $\mathbf{u}_{k}(\mathbf{s}_{i}) =$ vector of velocity responses at \mathbf{s}_{i} and time step k; 929 $\ddot{\mathbf{u}}_k(\mathbf{s}_i)$ = vector of acceleration responses at \mathbf{s}_i and time step k; 930 \mathbf{w}_k = sensor noise at time step k; \mathbf{x}_k = standard state vector at time step k; size is $pN \times 1$; 933 934 $\mathbf{x}_k^* = \text{TPM}$ state vector at time step k; size is $pN_{\alpha} \times 1$ and 936 $N_{\alpha} = N_O = M;$ \mathbf{y}_k = observation vector at time-step k (transposed row of 938 939 DSN data); size is $N_0 \times 1$; \mathbf{z}_k = modal state vector at time-step k; size is $pM \times 1$ and 940 942 $M \ll N;$ α = spatial subset corresponding to VPL; individually 943 945 indexed as $\alpha_1, \ldots, N_\alpha$; 946 $\beta^n() = B$ -spline function with degree n; 949 χ = uniformly spaced sensing node subset; individually 950 indexed as χ_1, \ldots, N_{χ} ; δ = general sensing node subset; individually indexed as 952 953 $\delta_1, \ldots, N_{\delta};$ Φ = mode shape matrix for *M* modes at all sensing nodes; 954 also known as $\Phi^{\langle M \rangle}(\mathbf{s})$; size is $N \times M$; 956

 Δt = sampling period in seconds;

 Φ_i = mode shape matrix for M modes at sensing nodes in subset i; also known as $\Phi^{\langle M \rangle}(\mathbf{s}_i)$; size is $N_i \times M$;

 $\mathbf{\eta}_k$ = forcing function at time-step k; size is $N \times 1$;

 \mathbf{v}_k = modal input at time-step k; size is $M \times 1$ and $M \ll N$;

 Ω = TPM mode shape regression term; size is $N_O \times M$ and $M = N_O$;

 $\Omega_{\text{sinc}} = sinc$ basis estimate for mode shape regression term; size is $N_O \times M$ and $M = N_O$; and

 $\Omega_{\text{spline}} = B$ -spline estimate of degree *n* for mode shape regression term; size is $N_O \times M$ and $M = N_O$.

References

- Abdel-Ghaffar, A., and Scanlan, R. H. (1985). "Ambient vibration studies of Golden Gate Bridge: 1. Suspended structure." J. Eng. Mech., 10 977 .1061/(ASCE)0733-9399(1985)111:4(463), 463–482. 978
 Abdel-Ghaffar, A. M. (1976). "Dynamic analyses of suspension bridge 979
- Abdel-Ghaffar, A. M. (1976). "Dynamic analyses of suspension bridge structures." *Final Rep. No. EERL-76-01*, Earthquake Engineering Research Laboratory, California Institute of Technology, Pasadena, CA.
- Aldroubi, A., Unser, M., and Eden, M. (1992). "Cardinal spline filters: Stability and convergence to the ideal sinc interpolator." *Signal Process.*, 28(2), 127–138.
- Andersen, P., Brinker, R., Peeters, B., De Roeck, G., Hermans, L., and Krämer, C. (1999). "Comparison of system identification methods using ambient bridge test data." *Int. Modal Analysis Conf.*, 1035–1041.
- Beutler, F. J. (1961). "Sampling theorems and bases in a Hilbert space." J. Inf. Control, 4(2), 97–117.
- Beutler, F. J. (1966). "Error-free recovery of signals from irregularly spaced samples." *SIAM Rev.*, 8(3), 328–335.
- Butzer, P. L., Engels, W., Ries, S., and Stens, R. L. (1986). "The Shannon sampling series and the reconstruction of signals in terms of linear, quadratic and cubic splines." *SIAM J. Appl. Math.*, 46(2), 299–323.
- Carder, D. S. (1937). "Observed vibrations of bridges." *Bull. Seismol. Soc. Am.*, 27(4), 267–303.
- Cerda, F., et al. (2012). "Indirect structural health monitoring in bridges: Scale experiments." *Proc.*, *7th Int. Conf. on Bridge Maintenance, Safety and Management*, Lago Di Como, 346–353.
- Chang, M., and Pakzad, S. N. (2012). "Modified natural excitation technique for stochastic modal identification." J. Struct. Eng., 10.1061/ (ASCE)ST.1943-541X.0000559, 1753–1762.
- Chang, M., and Pakzad, S. N. (2013). "Observer kalman filter identification for output-only systems using interactive structural modal identification toolsuite (SMIT)." J. Bridge Eng., 19(5), 1–11.
- Chang, M., and Pakzad, S. N. (2014). "Optimal sensor placement for modal identification of bridge systems considering number of sensing nodes." 1007
 J. Bridge Eng., 10.1061/(ASCE)BE.1943-5592.0000594, 04014019. 1008
- Chopra, A. K. (2007). *Dynamics of structures—Theory and applications to earthquake engineering*, Pearson, Prentice Hall, Upper Saddle River, NJ.
- Dorvash, S., Pakzad, S., Naito, C., Hodgson, I., and Yen, B. (2014a).
 "Application of state-of-the-art in measurement and data analysis techniques for vibration evaluation of a tall building." *Struct. Infrastruct.*1012
 1013
 1014
 1014
 Eng., 10(5), 654–669.
- Dorvash, S., Pakzad, S. N., and Labuz, E. L. (2014b). "Statistics based localized damage detection using vibration response." *Smart Struct. Syst.*, 14(2), 85–104.
- Gonzalez, A., O'Brien, E. J., and McGetrick, P. J. (2012). "Identification of damping in a bridge using a moving instrumented vehicle." *J. Sound Vib.*, 331(18), 4115–4131.
- Guo, H. Y., Zhang, L., Zhang, L. L., and Zhou, J. X. (2004). "Optimal placement of sensors for structural health monitoring using improved genetic algorithms." *Smart Mater. Struct.*, 13(3), 528–534.

Inaudi, D., and Glisic, B. (2010). "Long-range pipeline monitoring by distributed fiber optic sensing." J. Press. Vessel Technol., 132(1), 011701. 1026

Jagerman, D. L., and Fogel, L. J. (1956). "Some general aspects of the sampling theorem"." *IRE Trans. Inf. Theory*, 2(4), 139–146. 1028

969 961

958

961 963

964

966

968

960

971

973

974

975

980

981

982

983

984

985

986

987

988

989

990

991

992

993

994

995

996

997

998

999

1000

1001

1002

1003

1004

1005

1009

1010

1011

1016

1017

1018

1019

1020

1021

1022

1023

- Jerri, A. J. (1977). "Shannon sampling theorem—Its various extensions and applications: A tutorial review." *Proc. IEEE*, 65(11), 1565–1596.
- Juang, J.-N., and Pappa, R. S. (1984). "An eigensystem realization algorithm for modal parameter identification and model reduction." *NASA Langley Res. Center*, 8(5), 620–627.
- Juang, J.-N., and Phan, M. Q. (2001). Identification and control of mechanical systems, Cambridge University Press, Cambridge, U.K.
- Kotelnikov, W. A. (1933). "On the transmission capacity of the 'ether' and of cables in electrical communications." *Proc., 1st All-Union Conf. on the Technological Reconstruction of the Communications Sector and the Development of Low-Current Engineering*, Moscow.
- Lei, Y., et al. (2003). "Statistical damage detection using time series analysis on a structural health monitoring benchmark problem." *Proc.*,
 9th Int. Conf. on Applications of Statistics and Probability in Civil
 Engineering, 6–9.
- Lin, C. W., and Yang, Y. B. (2005). "Use of a passing vehicle to scan the fundamental bridge frequencies: An experimental verification." *Eng. Struct.*, 27(13), 1865–1878.
- Matarazzo, T. J., and Pakzad, S. N. (2014). "Modal identification of Golden Gate Bridge using pseudo mobile sensing data with STRIDE." *Dynamics of Civil Structures*, Vol. 4, Springer, 293–298.
- Matarazzo, T. J., and Pakzad, S. N. (2015a). "STRIDE for Structural identification using expectation maximization: Iterative output-only method for modal identification." *J. Eng. Mech.*, 10.1061/(ASCE)EM.1943-7889.0000951, in press.
- Matarazzo, T. J., and Pakzad, S. N. (2015b). "Structural modal identification for mobile sensing with missing data." *J. Eng. Mech.*, 10.1061/ (ASCE)EM.1943-7889.0001046, in press.
- Matarazzo, T. J., Shahidi, S. G., Chang, M., and Pakzad, S. N. (2015a).
 "Are today's SHM procedures suitable for tomorrow's BIGDATA?" *Structural Health Monitoring and Damage Detection*, Vol. 7, Springer,
 59–65.
- Matarazzo, T. J., Shahidi, S. G., and Pakzad, S. N. (2015b). "Exploring the efficiency of BIGDATA analyses in SHM" *Proc.*, 10th Int. Workshop on Structural Health Monitoring, DEStech, Toronto, ON, Canada.
- McGetrick, P. J., González, A., and O'Brien, E. J. (2009). "Theoretical investigation of the use of a moving vehicle to identify bridge dynamic parameters." *Insight—Non-destructive testing and condition monitoring*, 51(8), 433–438.
- McLamore, V. R., Hart, G. C., and Stubbs, I. R. (1971). "Ambient vibration of two suspension bridges." J. Struct. Div., 97(10), 2567–2582.
- Moheimani, S. O. R., Halim, D., and Fleming, A. J. (2003). Spatial control of vibration: Theory and experiments, A. Guran, C. Christov, M. Cloud,
 F. Pichler, and W. B. Zimmerman, eds., World Scientific, Singapore.
- P. Fichler, and W. B. Zhinherman, eds., world Scientific, Singapore.
 Oppenheim, A. V., Schafer, R. W., and Buck, J. R. (1999). *Discrete-time* signal processing, Prentice-Hall, Upper Saddle River, NJ.
- Pakzad, S. N., and Fenves, G. L. (2009). "Statistical analysis of vibration modes of a suspension bridge using spatially dense wireless sensor network." *J. Struct. Eng.*, 10.1061/(ASCE)ST.1943-541X.0000033, 863–872.

N.

- Pakzad, S. N., Fenves, G. L., Kim, S., and Culler, D. E. (2008). "Design and implementation of scalable wireless sensor network for structural monitoring." *J. Infastruct. Syst.*, 10.1061/(ASCE)1076-0342(2008)14: 1081 1(89), 89–101.
- Papadimitriou, C. (2004). "Optimal sensor placement methodology for parametric identification of structural systems." J. Sound Vib., 278(4–5), 923–947.
- Peeters, B., and De Roeck, G. (1999). "Reference-based stochastic subspace identification for output-only modal analysis." *Mech. Syst. Signal Process.*, 13(6), 855–878.

1086

1087

1088

1097

1098

1106

1107

1112

1113

1114

1115

1119

1120

- Rainer, J. H., and Selst, V. A. (1976). "Dynamic properties of Lions' Gate suspension bridge." ASCE/EMD Specialty Conf.: Dynamic Response of Structures: Instrumentation, Testing Methods, and System Identification, Los Angeles, CA, 243–252.
- Shahidi, S. G., Pakzad, S. N., Ricles, J. M., Martin, J. R., Olgun, C. G., and Godfrey, E. A. (2015). "Behavior and damage of the Washington monument during the 2011 Mineral, Virginia, earthquake." *Geol. Soc.* 1095 *Am.*, 2509(13), 235–252. 1096
- Shannon, C. E. (1998). "Communication in the presence of noise." *Proc. IEEE*, 86(2), 447–457.
- Smyth, A., and Wu, M. (2007). "Multi-rate Kalman filtering for the data fusion of displacement and acceleration response measurements in dynamic system monitoring." *Mech. Syst. Sig. Process.*, 21(2), 706–723. 1101
- Smyth, A. W., Pei, J.-S., and Masri, S. F. (2003). "System identification of the Vincent Thomas suspension bridge using earthquake records by system identification of the Vincent Thomas suspension bridge using earthquake records." *Earthquake Eng. Struct. Dyn.*, 32(3), 339–367.
- Stenger, F. (1976). "Approximations via Whittaker's cardinal function." J. Approx. Theory, 17(3), 222–240.
- Trifunac, M. D. (1970). "Wind and microtremor induced vibrations of a twenty-two story steel frame building." *Final Rep. No. EERL-70-02*, Earthquake Engineering Research Laboratory, California Institute of Technology, Pasadena, CA.
- Unser, M. (1999). "Splines: A perfect fit for signal and image processing." *IEEE Signal Process. Mag.*, 16(6), 22–38.
- Vincent, G. S. (1962). "Golden Gate Bridge vibration studies." Trans. ASCE, 127(2), 667–707.
- Whittaker, E. T. (1915). "On the functions which are represented by the expansions of interpolation-theory." *Proc. Roy. Soc. Edinburgh*, 1117 35(1915), 181–194.
- Whittaker, J. M. (1928). "The 'Fourier' theory of the cardinal function." Proc. Edinburgh Math. Soc., 1(3), 169–176.
- Yang, Y.-B., Lin, C. W., and Yau, J. D. (2004). "Extracting bridge frequencies from the dynamic response of a passing vehicle." *J. Sound Vib.*, 1122 272(3–5), 471–493.
- Zhu, D., Guo, J., Cho, C., Wang, Y., and Lee, K. (2012). "Wireless mobile 1124 sensor network for the system identification of a space frame bridge." *IEEE/ASME Trans. Mechatron.*, 17(3), 499–507. 1126